



WESTERN REGIONAL GUIDE

California

**Technical Data for Joists,
Headers, Beams, Rim Board,
Columns, and Dimension**

ENGINEERED WOOD PRODUCTS

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I-Joists and LVL

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Evaluation Reports

I-JOIST EVALUATION REPORTS

Building Code / Authority	Evaluation Service / Department	Report No.
International Building Code International Residential Code	APA - The Engineered Wood Association	PR-L262
	ICC-ES	ESR-1225
National Building Code of Canada	CCMC	13470-R
U.S. Dept. of Housing and Urban Development (HUD)	Manufactured Housing and Standards Division	MR 1341
City of Los Angeles	Department of Building and Safety (LADBS)	RR 25450

GREEN VERIFICATION REPORT

Subject	Certification Body	Report No.
Green Verification	APA - The Engineered Wood Association	GR-L262
Low Formaldehyde Emissions	APA - The Engineered Wood Association	PR-E730

LVL EVALUATION REPORTS

Building Code / Authority	Evaluation Service / Department	Report No.
International Building Code International Residential Code	APA - The Engineered Wood Association	PR-L233
	ICC-ES / APA	ESR-2909
National Building Code of Canada	CCMC	13006-R
U.S. Dept. of Housing and Urban Development (HUD)	Manufactured Housing and Standards Division	MR 1310b
City of Los Angeles	Department of Building and Safety (LADBS)	RR 25448

GREEN VERIFICATION REPORT

Subject	Certification Body	Report No.
Green Verification	APA - The Engineered Wood Association	GR-L233
Low Formaldehyde Emissions	APA - The Engineered Wood Association	PR-E720

For information about Pacific Woodtech's Evaluation Reports, please visit www.pacificwoodtech.com.

Safety and Construction Precautions

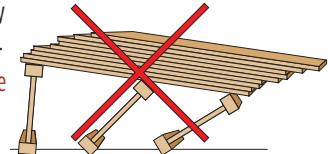
Installation



- Walking on the joists should not be permitted until they are properly braced.
- All hangers, rim boards, rim joists and blocking at the end supports of the joists must be installed and nailed properly.
- During installation, a minimum of 1 x 4 temporary bracing is required.
- Bracing members should be spaced at 8' - 0" o.c. and nailed to each joist with two 8d nails (10d box nails if bracing thickness exceeds 1").
- Lap bracing ends and anchor them to temporary or permanent sheathing nailed to the first 4' of joists at the end of the bay or a braced end wall.

- Do not cut, drill or notch flanges.
- The ends of cantilevers must be temporarily braced on both the top and bottom flanges.
- Never overload sheathed joists with loads that exceed design loads.
- Only remove the bracing as the sheathing is attached.
- Engineered wood products should be used in dry conditions only.
- When stacking construction material, stack only over beams or walls, NOT on unsheathed joists.

These are general recommendations and, in some cases, additional precautions may be required.



Storage and Handling Guidelines

Storage

- Installation guidelines from Pacific Woodtech will be included with every shipment of trademarked PWI joists to job sites.
- Store bundles upright on a smooth, level, well-drained and supportive surface.
- Always stack and handle I-joists in the upright position only.
- Bundles should not be in contact with the ground.
- Place 2x or LVL spacers (at a maximum of 10' apart) between bundles and the ground and bundles stored on top of one another.
- Bundles should remain wrapped, strapped and protected from the weather until time of installation.

LVL Sealer

Pacific Woodtech's LVL has a wax-based sealer specifically formulated for laminated veneer lumber to help protect it from weather-related issues during

storage and construction. LVL is very dry when it is produced. It will absorb moisture and grow in size slightly as it acclimates to the climate. The sealer helps to reduce the rate of moisture absorption and increases protection from UV rays. However, it is not meant for protection from long-term or high concentrations of moisture exposure.

Handling

Never use or field repair a damaged I-joist.

- All handling of joists with a forklift or crane should be done carefully.
- Joists should remain vertical during handling.
- Avoid excessive bowing during all phases of handling and installation (i.e., measuring, sawing or placement).
- Damage may result if the joist or beam is twisted or a load is applied to it while it's lying flat.

System Performance

Traditionally, floor vibration has not been an issue with a well-designed and constructed floor. The model code-required serviceability deflection requirements of span/360 for live load and span/240 for total load have long served to keep code-conforming floors stiff enough to minimize vibration-related problems. These deflection requirements were based on the use of traditional lumber framing and prevailing architectural norms. Spans in traditional lumber-framed structures seldom exceeded 14-16 feet.

With engineered wood products, however, designers are no longer limited by the capacities and lengths of traditional lumber structural elements. Spans unheard of just a few years ago are now common with engineered wood products. The traditional deflection limits may no longer be appropriate for the longer spans made possible by engineered wood products. For this reason, APA has voluntarily adopted a live load deflection criteria that is 33% stiffer than that required in the current model building codes. This deflection criteria was selected for increase because vibration loads are caused by transient or live loads, most often by people moving about the floor itself.

By increasing the stiffness of the floor—using span/480 requirements instead of the more traditional span/360—the vibrations caused by a thundering herd of youngsters can be more easily tolerated. Designing the ideal floor is not, however, an exact science. Because one of the benefits of a wood floor is its ability to cushion footfalls, it is not desirable to make every floor overly stiff.

As usual, a one-size solution does not fit all. The selection of span/480 as a serviceability requirement is a compromise. It provides a substantial decrease in floor vibration with a minimal cost penalty without making the floor so stiff that comfort is compromised.

Researchers have proposed a number of additional methods that can be used to reduce floor vibration even further. These methods include:

- Gluing the wood structural panel floor to the PWI joists
- Attaching wood structural panels or gypsum board to the bottom of the PWI floor joists
- Decreasing the PWI floor joist spacing by one increment based on allowable span
- Using full-depth blocking at regular intervals between all of the PWI floor joists over the entire floor
- Adding concrete topping over the floor sheathing

By far the most practical and most economical way to further increase the stiffness of your floor when using PWI joists is to select the most economical joist from our allowable span tables and then maintain the same joist designation but upgrade to the next net depth.



Proposed Conversion Chart

PACIFIC WOODTECH CORPORATION ENGINEERED WOOD CONVERSION CHART*

I-JOIST	Pacific Woodtech†	Trus Joist	Boise	Roseburg	Red Built	LP	LPss
Up to 24"	PWI / 20	TJI / 110	BCI / 5000	RFPI /20	DNA	LPI / 450	LPI / 20
	PWI / 45	TJI / 210	BCI / 6000	RFPI / 400	DNA	LPI / 530	LPI / 20
	PWI / 60	TJI / 230	BCI / 6500	RFPI / 40	Red-I 45	LPI / 530	LPI / 32
	PWI / 70	TJI / 360	BCI / 60	RFPI / 70	DNA	LPI / 36	
	PWI / 77	DNA	DNA	RFPI / 700	Red-I 65	DNA	LPI / 42
	PWI / 90	TJI / 560	BCI / 90	RFPI / 90	Red-I 90	LPI / 56	

STRUCTURAL COMPOSITE LUMBER (LVL AND PARALLAM [PSL])

BEAM / HEADER	Pacific Woodtech†	Trus Joist	Boise	Roseburg	Red Built	LP
1¾"	LVL (1.5E)	LSL (1.55E)	LVL (1.7E)	LVL (1.5E)	LVL (1.5E)	LSL (1.55E)
	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)
3½"	LVL (2.0E)	LSL (1.55E)	LVL (1.7E)	LVL (1.5E)	LVL (1.5E)	LSL (1.55E)
	LVL (2.0E)	PSL (2.0E)	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)	LVL (2.0E)
3½" - 7"	LVL (2.0E / 2.2E**)	PSL (2.0E / 2.2E)	LVL (2.0E)	LVL (2.0E)	DNA	LVL (2.0E)

RIM BOARD	Pacific Woodtech†	Trus Joist	Boise	Roseburg	Red Built	LP
1¼" - 1¾"	LVL Rim Board	LSL Rim Board	LVL Rim Board	LVL Rim Board	LVL Rim Board	LSL Rim Board

DIMENSION	Pacific Woodtech†	Trus Joist	Boise	Roseburg	Red Built	LP
Various Sizes	LVL	LSL	LVL	LVL	LVL	LSL

COLUMNS	Pacific Woodtech†	Trus Joist	Boise	Roseburg	Red Built	LP
Various Sizes	LVL	PSL	LVL	LVL	LVL	LVL

* Please note: This conversion chart is intended to provide a reference to similar strength / performance characteristics by respective manufacturers per current (04/2017) regional specifier guides.

** 2.2E, a limited stock item, is available upon request.

† Code reports for Pacific Woodtech Corporation: I-joist (ESR-1225, City of LA 25450); LVL (ESR-2909, City of LA 25448)

Substitutions are subject to a review committee, which may include the project structural engineer.

Please contact your representative for assistance with all conversions of the Red-I 65. Some cases may require upgrade to PWI-90.

Questions? Please contact JD Dombeck at 425.890.3996

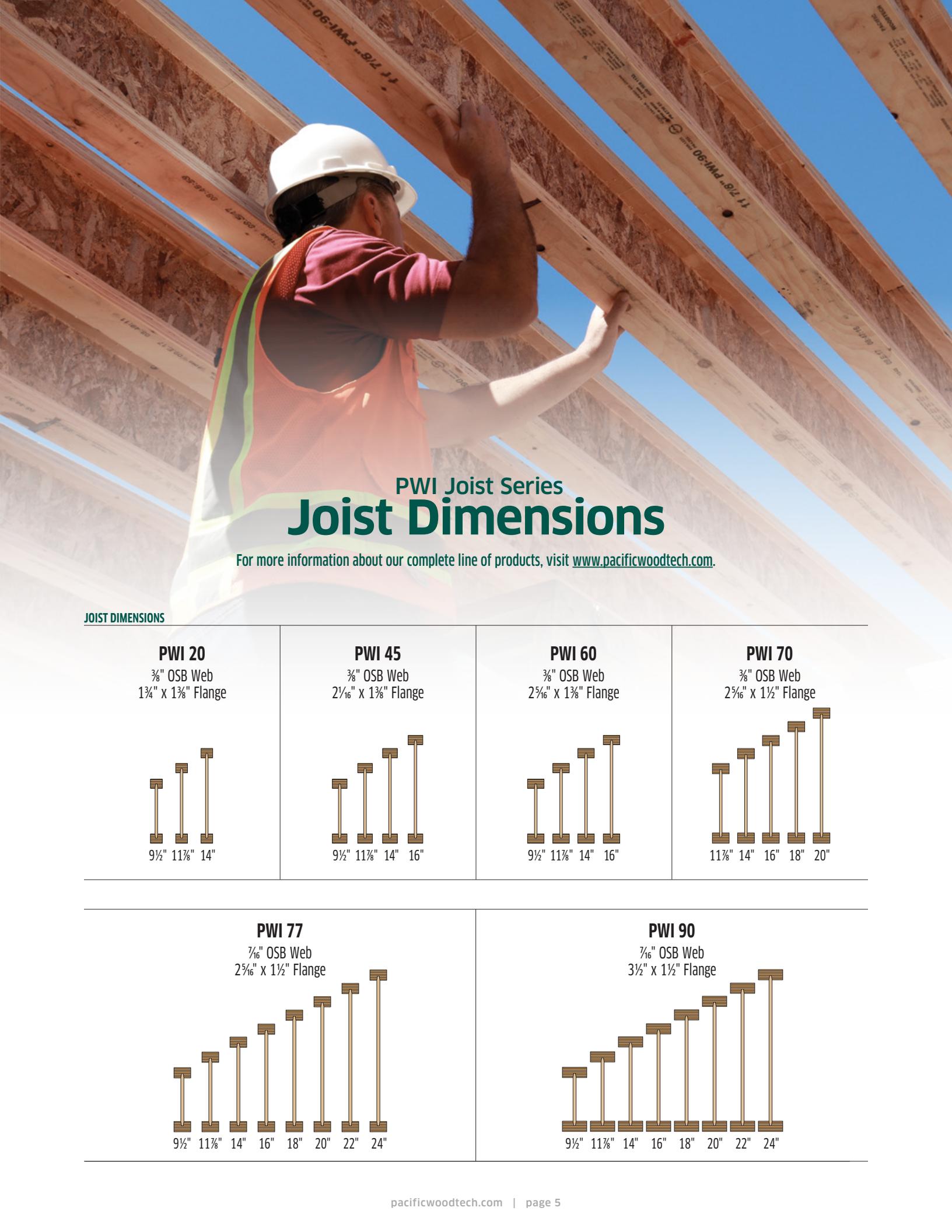


Product Warranty

Pacific Woodtech Corporation warrants that its products, as manufactured, will be free from manufacturing errors or defects in workmanship and material.

In addition, provided the product, as manufactured, is stored, handled, installed and used correctly, Pacific Woodtech Corporation warrants the adequacy of its design.

This warranty is backed by the full resources of Pacific Woodtech Corporation and by underwritten product liability insurance.



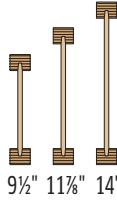
PWI Joist Series **Joist Dimensions**

For more information about our complete line of products, visit www.pacificwoodtech.com.

JOIST DIMENSIONS

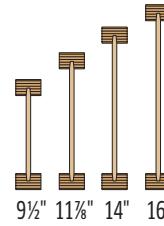
PWI 20

$\frac{3}{8}$ " OSB Web
 $1\frac{3}{4}$ " x $1\frac{1}{8}$ " Flange



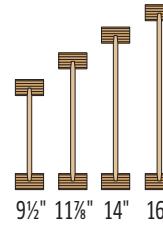
PWI 45

$\frac{3}{8}$ " OSB Web
 $2\frac{1}{16}$ " x $1\frac{1}{8}$ " Flange



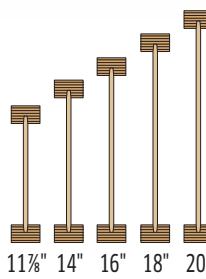
PWI 60

$\frac{3}{8}$ " OSB Web
 $2\frac{5}{16}$ " x $1\frac{1}{8}$ " Flange



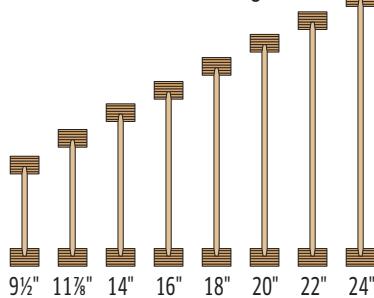
PWI 70

$\frac{3}{8}$ " OSB Web
 $2\frac{5}{16}$ " x $1\frac{1}{2}$ " Flange



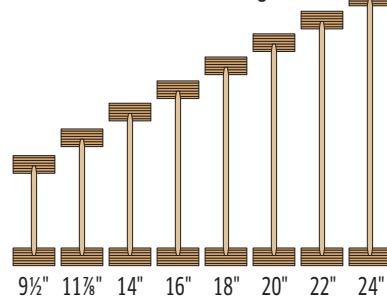
PWI 77

$\frac{7}{16}$ " OSB Web
 $2\frac{5}{16}$ " x $1\frac{1}{2}$ " Flange



PWI 90

$\frac{7}{16}$ " OSB Web
 $3\frac{1}{2}$ " x $1\frac{1}{2}$ " Flange



PWI Joist Series

Reference Design Values

REFERENCE DESIGN VALUES⁽¹⁾

Joist Series	Joist Depth	EI ⁽²⁾ (x 10 ⁶ lb-in ²)	k ⁽³⁾ (x 10 ⁶ lb)	M ⁽⁴⁾ (ft-lb)	V ⁽⁵⁾ (lb)	ER ⁽⁶⁾ (lb)	IR ⁽⁷⁾ (lb)	Vertical Load ⁽⁸⁾ (plf)	Weight (plf)
PWI 20	9½"	145	4.94	2520	1330	915	1990	2000	2.0
	11⅞"	253	6.18	3265	1705	915	1990	2000	2.3
	14"	373	7.28	3890	1955	915	1990	2000	2.5
PWI 45	9½"	193	4.94	3345	1330	980	2240	2000	2.3
	11⅞"	330	6.18	4315	1705	980	2250	2000	2.5
	14"	486	7.28	5140	1955	980	2250	2000	2.8
	16"	665	8.32	5880	2190	980	2250	2000	3.0
PWI 60	9½"	231	4.94	3780	1330	1080	2240	2000	2.4
	11⅞"	396	6.18	4900	1705	1080	2330	2000	2.7
	14"	584	7.28	5895	1955	1080	2330	2000	2.9
	16"	799	8.32	6835	2190	1080	2330	2000	3.2
PWI 70	11⅞"	440	6.18	6730	1705	1160	2460	2000	2.8
	14"	644	7.28	8030	1955	1160	2460	2000	3.1
	16"	873	8.32	9200	2190	1160	2460	2000	3.3
	18"	1141	9.36	10355	2425	1160	2460	1450	3.5
	20"	1447	10.40	11495	2660	1160	2460	1450	3.7
PWI 77	9½"	261	6.08	5155	1430	1285	2695	2400	2.7
	11⅞"	442	7.60	6675	1925	1285	2695	2400	3.0
	14"	648	8.96	7960	2125	1285	2695	2400	3.3
	16"	881	10.24	9120	2330	1285	2695	2400	3.5
	18"	1152	11.52	10265	2535	1285	2695	1800	3.8
	20"	1463	12.80	11395	2740	1285	2695	1800	4.1
	22"	1815	14.08	12520	2935	2390 ⁽⁹⁾	4125 ⁽⁹⁾	1300	4.3
	24"	2209	15.36	13630	3060	2390 ⁽⁹⁾	4125 ⁽⁹⁾	1300	4.6
PWI 90	9½"	392	6.08	7915	1430	1400	2860	2400	3.6
	11⅞"	661	7.60	10255	1925	1400	3355	2400	3.9
	14"	965	8.96	12235	2125	1400	3355	2400	4.2
	16"	1306	10.24	14020	2330	1400	3355	2400	4.5
	18"	1703	11.52	15780	2535	1400	3355	1800	4.7
	20"	2155	12.80	17520	2740	1400	3355	1800	5.0
	22"	2664	14.08	19245	2935	2400 ⁽⁹⁾	4605 ⁽⁹⁾	1300	5.3
	24"	3232	15.36	20955	3060	2400 ⁽⁹⁾	4605 ⁽⁹⁾	1300	5.5

1. Values apply to normal load duration. All values except EI, k and Vertical Load may be adjusted for other load durations as permitted by the code.

2. Bending stiffness (EI).

3. Coefficient of shear deflection (k). Use Equations 1 or 2 to calculate uniform load or center point load deflections in a simple-span application.

Uniform Load:

$$[1] \delta = \frac{5w\ell^4}{384EI} + \frac{w\ell^2}{k}$$

Center Point Load:

$$[2] \delta = \frac{P\ell^3}{48EI} + \frac{2P\ell}{k}$$

Where:

$$\delta = \text{calculated deflection [in]}$$

$$w = \text{uniform load [lb/in]}$$

$$\ell = \text{design span [in]}$$

P = concentrated load [lb]

EI = bending stiffness of the I-joist [lb-in²]

k = coefficient of shear deflection [lb]

4. Moment capacity (M). The tabulated values shall not be increased by any code-allowed repetitive member factor.

5. Shear capacity (V).

6. End reaction capacity (ER) of the I-joist without web stiffeners and a minimum bearing length of 1½ inches.

7. Intermediate reaction capacity (IR) of the I-joist without web stiffeners and a minimum bearing length of 3½ inches.

8. Blocking panel and rim joist vertical load capacity.

9. Web stiffeners required. See *Web Stiffener Requirements* on page 11.

Floor Spans

ALLOWABLE RESIDENTIAL FLOOR SPANS – 40 PSF LIVE LOAD AND 20 PSF DEAD LOAD

Joist Series	Joist Depth	Simple Spans				Multiple Spans				Simple or Multiple Spans			
		12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.
PWI 20	9½"	16'-7"	15'-3"	14'-4"	12'-9"	18'-1"	15'-7"	14'-3"	12'-8"	16'-7"	15'-3"	14'-3"	12'-8"
	11½"	19'-11"	17'-11"	16'-4"	14'-7"	20'-7"	17'-10"	16'-3"	13'-0"	19'-11"	17'-10"	16'-3"	13'-0"
	14"	22'-7"	19'-6"	17'-10"	15'-1"	22'-6"	19'-6"	16'-4"	13'-0"	22'-6"	19'-6"	16'-4"	13'-0"
PWI 45	9½"	18'-0"	16'-5"	15'-6"	14'-6"	20'-0"	18'-0"	16'-5"	14'-8"	18'-0"	16'-5"	15'-6"	14'-6"
	11½"	21'-5"	19'-7"	18'-6"	16'-2"	23'-9"	20'-6"	18'-6"	14'-9"	21'-5"	19'-7"	18'-6"	14'-9"
	14"	24'-4"	22'-3"	20'-3"	16'-2"	25'-11"	22'-3"	18'-6"	14'-9"	24'-4"	22'-3"	18'-6"	14'-9"
	16"	27'-0"	24'-1"	20'-3"	16'-2"	27'-9"	22'-3"	18'-6"	14'-9"	27'-0"	22'-3"	18'-6"	14'-9"
PWI 60	9½"	18'-11"	17'-3"	16'-3"	15'-2"	21'-1"	19'-2"	17'-6"	14'-8"	18'-11"	17'-3"	16'-3"	14'-8"
	11½"	22'-7"	20'-7"	19'-5"	17'-10"	25'-2"	21'-11"	19'-2"	15'-3"	22'-7"	20'-7"	19'-2"	15'-3"
	14"	25'-8"	23'-5"	22'-0"	17'-10"	27'-9"	23'-0"	19'-2"	15'-3"	25'-8"	23'-0"	19'-2"	15'-3"
	16"	28'-6"	25'-11"	22'-4"	17'-10"	29'-11"	23'-0"	19'-2"	15'-3"	28'-6"	23'-0"	19'-2"	15'-3"
PWI 70	11½"	23'-4"	21'-3"	20'-1"	18'-8"	25'-11"	23'-8"	20'-3"	16'-2"	23'-4"	21'-3"	20'-1"	16'-2"
	14"	26'-5"	24'-1"	22'-9"	19'-2"	29'-6"	24'-4"	20'-3"	16'-2"	26'-5"	24'-1"	20'-3"	16'-2"
	16"	29'-3"	26'-8"	24'-0"	19'-2"	32'-6"	24'-4"	20'-3"	16'-2"	29'-3"	24'-4"	20'-3"	16'-2"
	18"	32'-0"	28'-10"	24'-0"	19'-2"	32'-6"	24'-4"	20'-3"	16'-2"	32'-0"	24'-4"	20'-3"	16'-2"
	20"	34'-8"	28'-10"	24'-0"	19'-2"	32'-6"	24'-4"	20'-3"	16'-2"	32'-6"	24'-4"	20'-3"	16'-2"
PWI 77	9½"	19'-8"	18'-0"	17'-0"	15'-10"	21'-11"	20'-1"	18'-11"	17'-8"	19'-8"	18'-0"	17'-0"	15'-10"
	11½"	23'-5"	21'-5"	20'-3"	18'-11"	26'-2"	23'-10"	22'-2"	17'-8"	23'-5"	21'-5"	20'-3"	17'-8"
	14"	26'-7"	24'-4"	23'-0"	21'-3"	29'-8"	26'-8"	22'-2"	17'-8"	26'-7"	24'-4"	22'-2"	17'-8"
	16"	29'-6"	26'-11"	25'-5"	21'-3"	32'-11"	26'-8"	22'-2"	17'-8"	29'-6"	26'-8"	22'-2"	17'-8"
	18"	32'-3"	29'-5"	26'-7"	21'-3"	35'-8"	26'-8"	22'-2"	17'-8"	32'-3"	26'-8"	22'-2"	17'-8"
	20"	34'-11"	31'-10"	26'-7"	21'-3"	35'-8"	26'-8"	22'-2"	17'-8"	34'-11"	26'-8"	22'-2"	17'-8"
	22"	37'-6"	34'-3"	32'-1"	28'-8"	40'-7"	35'-1"	32'-0"	27'-3"	37'-6"	34'-3"	32'-0"	27'-3"
	24"	40'-1"	36'-7"	33'-6"	29'-11"	42'-4"	36'-8"	33'-5"	27'-3"	40'-1"	36'-7"	33'-5"	27'-3"
PWI 90	9½"	22'-3"	20'-3"	19'-1"	17'-9"	24'-9"	22'-6"	21'-3"	18'-10"	22'-3"	20'-3"	19'-1"	17'-9"
	11½"	26'-5"	24'-1"	22'-8"	21'-2"	29'-6"	26'-10"	25'-3"	22'-1"	26'-5"	24'-1"	22'-8"	21'-2"
	14"	30'-0"	27'-4"	25'-9"	23'-2"	33'-5"	30'-5"	27'-8"	22'-1"	30'-0"	27'-4"	25'-9"	22'-1"
	16"	33'-2"	30'-3"	28'-6"	23'-2"	37'-0"	33'-3"	27'-8"	22'-1"	33'-2"	30'-3"	27'-8"	22'-1"
	18"	36'-3"	33'-0"	29'-0"	23'-2"	40'-6"	33'-3"	27'-8"	22'-1"	36'-3"	33'-0"	27'-8"	22'-1"
	20"	39'-3"	34'-10"	29'-0"	23'-2"	43'-9"	33'-3"	27'-8"	22'-1"	39'-3"	33'-3"	27'-8"	22'-1"
	22"	42'-1"	38'-4"	36'-2"	33'-8"	47'-0"	42'-9"	38'-1"	30'-5"	42'-1"	38'-4"	36'-2"	30'-5"
	24"	44'-11"	40'-11"	38'-7"	35'-11"	50'-2"	45'-6"	38'-1"	30'-5"	44'-11"	40'-11"	38'-1"	30'-5"

Notes:

1. Table values apply to uniformly loaded, residential floor joists.
2. Span is measured from face to face of supports.
3. Deflection is limited to L/240 at total load and L/480 at live load.
4. Table values are based on glued and nailed sheathing panels (23/32" for 24" o.c., 19/32" otherwise). Use an ASTM D3498 adhesive in accordance with the manufacturer's recommendations. Reduce spans by 12" if sheathing is nailed only.
5. Provide at least 1¾" of bearing length at end supports and 3½" at intermediate supports.
6. Provide lateral restraint at supports (e.g. blocking panels, rim board) and along the compression flange of each joist (e.g. floor sheathing, gypsum board ceiling).
7. Use sizing software or consult a professional engineer to analyze conditions outside the scope of this table (e.g. commercial floors, different bearing conditions, concentrated loads) or for multiple span joists if the length of any span is less than half the length of an adjacent span.
8. Web stiffeners are required at all supports for 22" and 24" joists. See *Web Stiffener Requirements* on page 11 for more details.

How to Use Floor Span Tables

1. Choose the appropriate live and dead load combination as well as a joist spacing.
2. Scan down the spacing column to find a span that exceeds the design span.
3. Scan to the left from that span to determine the joist size required.

Floor Loads

SIMPLE-SPAN JOIST—ALLOWABLE UNIFORM FLOOR LOAD (PLF)

Joist Span (ft)	PWI 20						PWI 45						PWI 60													
	9½"		11¾"		14"		9½"		11¾"		14"		16"		9½"		11¾"		14"		16"					
	Live L/480	Total 100%																								
6	-	305	-	305	-	305	-	327	-	327	-	327	-	327	-	360	-	360	-	360	-	360				
7	-	261	-	261	-	261	-	280	-	280	-	280	-	280	-	309	-	309	-	309	-	309				
8	-	229	-	229	-	229	-	245	-	245	-	245	-	245	-	270	-	270	-	270	-	270				
9	185	203	-	203	-	203	-	218	-	218	-	218	-	218	-	240	-	240	-	240	-	240				
10	139	183	-	183	-	183	177	196	-	196	-	196	-	196	205	216	-	216	-	216	-	216				
11	107	166	-	166	-	166	137	178	-	178	-	178	-	178	160	196	-	196	-	196	-	196				
12	84	140	141	153	-	153	108	163	-	163	-	163	-	163	127	180	-	180	-	180	-	180				
13	67	119	113	141	-	141	87	151	143	151	-	151	-	151	102	166	-	166	-	166	-	166				
14	54	103	92	131	-	131	71	137	117	140	-	140	-	140	83	154	137	154	-	154	-	154				
15	45	89	76	116	109	122	58	116	96	131	-	131	-	131	68	134	113	144	-	144	-	144				
16	37	74	63	102	91	114	48	97	81	123	116	123	-	123	57	114	95	135	-	135	-	135				
17		53	90	77	108			68	115	98	115		115			80	127	115	127		127		127			
18		45	81	66	96			58	107	83	109	-	109			68	120	98	120	-	120		120			
19		39	72	56	86			50	96	72	103	96	103			59	109	85	114	113	114		114			
20		33	65	48	78			43	86	62	98	83	98			51	98	73	108	98	108		108			
21				42	71					54	93	73	93					64	103	86	103		103			
22				37	64					47	85	64	89					56	97	75	98		98			
23				32	59					42	78	56	85					49	89	67	94		94			
24				29	54					37	71	50	82					44	82	59	90		90			
25												44	75							53	86				86	
26												40	70								47	81				81
27												35	65								42	75				75
28												32	60								38	70				70
29																										
30																										
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42																										

Notes:

1. Table values apply to uniformly loaded floor joists.
2. Span is measured to the center of each support.
3. The values in the Total columns are based on an L/240 total load deflection limit. Building codes typically require L/360 for live load. Experience has shown that a live load deflection limit of L/480 at 40 psf for residential floors does a better job than L/360 of meeting most performance expectations.
4. Table values do not account for stiffness added by glued or nailed sheathing.
5. Provide at least 1¼" of bearing length at end supports and 3½" at intermediate supports.
6. Provide lateral restraint at supports (e.g. blocking panels, rim board) and along the compression flange of each joist (e.g. floor sheathing, gypsum board ceiling).
7. Use sizing software or consult a professional engineer to analyze conditions outside the scope of this table (e.g. different bearing lengths, concentrated loads) or for multiple span joists if the length of any span is less than half the length of an adjacent span.

How to Use Floor Load Tables

1. Choose a joist spacing and convert the live and total design loads specified in pounds per square foot (psf) to joist loads in pounds per lineal foot (plf).
Joist Spacing [ft] x Design Load [psf] = Joist Load [plf]

JOIST LOAD (PLF)

Joist Spacing	Design Load (psf)									
	Inches	Feet	20	30	40	50	60	70	80	90
12	1	20	30	40	50	60	70	80	90	100
16	1.33	27	40	53	67	80	93	106	120	133
19.2	1.6	32	48	64	80	96	112	128	144	160
24	2	40	60	80	100	120	140	160	180	200

2. Choose a span and scan across the Span row to find a joist size with sufficient Live and Total load capacities. Both requirements must be satisfied. When no value is shown in a Live column, Total load governs.
3. Web stiffeners are required at all supports for 22" and 24" joists. See *Web Stiffener Requirements* on page 11 for more details.

Floor Loads

SIMPLE-SPAN JOIST—ALLOWABLE UNIFORM FLOOR LOAD (PLF)

Joist Span (ft)	PWI 70										PWI 77																
	11%"		14"		16"		18"		20"		9½"		11½"		14"		16"		18"		20"		22"		24"		
	Live L/480	Total 100% L/480																									
6	-	387	-	387	-	387	-	387	-	387	-	428	-	428	-	428	-	428	-	428	-	428	-	649	-	649	
7	-	331	-	331	-	331	-	331	-	331	-	367	-	367	-	367	-	367	-	367	-	367	-	556	-	556	
8	-	290	-	290	-	290	-	290	-	290	-	321	-	321	-	321	-	321	-	321	-	321	-	487	-	487	
9	-	258	-	258	-	258	-	258	-	258	-	286	-	286	-	286	-	286	-	286	-	286	-	432	-	432	
10	-	232	-	232	-	232	-	232	-	232	236	257	-	257	-	257	-	257	-	257	-	257	-	389	-	389	
11	-	211	-	211	-	211	-	211	-	211	183	234	-	234	-	234	-	234	-	234	-	234	-	354	-	354	
12	-	193	-	193	-	193	-	193	-	193	145	214	-	214	-	214	-	214	-	214	-	214	-	324	-	324	
13	-	178	-	178	-	178	-	178	-	178	116	198	189	198	-	198	-	198	-	198	-	198	-	299	-	299	
14	149	166	-	166	-	166	-	166	-	166	95	184	155	184	-	184	-	184	-	184	-	184	-	278	-	278	
15	124	155	-	155	-	155	-	155	-	155	78	156	128	171	-	171	-	171	-	171	-	171	-	259	-	259	
16	104	145	-	145	-	145	-	145	-	145	65	130	107	161	-	161	-	161	-	161	-	161	-	243	-	243	
17	88	136	125	136	-	136	-	136	-	136	-	-	90	151	129	151	-	151	-	151	-	151	-	229	-	229	
18	75	129	107	129	-	129	-	129	-	129	-	-	77	143	110	143	-	143	-	143	-	143	-	216	-	216	
19	64	122	92	122	-	122	-	122	-	122	-	-	66	132	95	135	127	135	-	135	-	135	-	205	-	205	
20	56	112	80	116	106	116	-	116	-	116	-	-	57	114	82	129	110	129	-	129	-	129	-	195	-	195	
21	-	-	70	110	93	110	-	110	-	110	-	-	-	-	-	71	122	96	122	-	122	-	122	-	185	-	185
22	-	-	61	105	82	105	105	105	-	105	-	-	-	-	-	63	117	84	117	108	117	-	117	166	177	-	177
23	-	-	54	101	72	101	93	101	-	101	-	-	-	-	-	55	110	74	112	96	112	-	112	147	169	-	169
24	-	-	48	96	64	97	82	97	-	97	-	-	-	-	-	49	98	66	107	85	107	106	107	130	162	157	162
25	-	-	-	-	57	93	73	93	92	93	-	-	-	-	-	-	58	103	75	103	95	103	116	156	140	156	
26	-	-	-	-	51	89	66	89	82	89	-	-	-	-	-	-	52	99	68	99	85	99	104	148	125	150	
27	-	-	-	-	46	86	59	86	74	86	-	-	-	-	-	-	47	94	61	95	76	95	94	137	113	144	
28	-	-	-	-	41	82	53	83	67	83	-	-	-	-	-	-	42	84	55	92	69	92	84	128	102	139	
29	-	-	-	-	-	48	80	61	80	-	-	-	-	-	-	-	49	89	62	89	76	119	92	130			
30	-	-	-	-	-	44	77	55	77	-	-	-	-	-	-	-	45	86	56	86	69	111	84	121			
31	-	-	-	-	-	40	75	50	75	-	-	-	-	-	-	-	41	81	51	83	63	104	76	113			
32	-	-	-	-	-	-	-	-	-	-	46	73	-	-	-	-	-	-	-	47	80	58	98	70	106		
33	-	-	-	-	-	-	-	-	-	-	42	70	-	-	-	-	-	-	-	43	78	53	92	64	100		
34	-	-	-	-	-	-	-	-	-	-	38	68	-	-	-	-	-	-	-	39	76	48	87	59	94		
35	-	-	-	-	-	-	-	-	-	-	35	66	-	-	-	-	-	-	-	36	72	45	82	54	89		
36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	41	77	50	84			
37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	38	73	46	80			
38	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	35	69	42	76			
39	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	39	72				
40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	37	68				
41	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	34	65				
42	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	62				

Notes:

1. Table values apply to uniformly loaded floor joists.
2. Span is measured to the center of each support.
3. The values in the Total columns are based on an L/240 total load deflection limit. Building codes typically require L/360 for live load. Experience has shown that a live load deflection limit of L/480 at 40 psf for residential floors does a better job than L/360 of meeting most performance expectations.
4. Table values do not account for stiffness added by glued or nailed sheathing.
5. Provide at least 1¼" of bearing length at end supports and 3½" at intermediate supports.
6. Provide lateral restraint at supports (e.g. blocking panels, rim board) and along the compression flange of each joist (e.g. floor sheathing, gypsum board ceiling).
7. Use sizing software or consult a professional engineer to analyze conditions outside the scope of this table (e.g. different bearing lengths, concentrated loads) or for multiple span joists if the length of any span is less than half the length of an adjacent span.

PSF TO PLF CONVERSION - LOAD IN POUNDS PER LINEAL FOOT (PLF)

O.C. Spacing	Load in Pounds per Square Foot (psf)												
Inches	Feet	20	25	30	35	40	45	50	55	60	65	70	75
12	1.00	20	25	30	35	40	45	50	55	60	65	70	75
16	1.33	27	33	40	47	53	60	67	73	80	87	93	100
19.2	1.60	32	40	48	56	64	72	80	88	96	104	112	120
24	2.00	40	50	60	70	80	90	100	110	120	130	140	150

O.c. spacing (ft) x load (psf) = load (plf)

Floor Loads

SIMPLE-SPAN JOIST—ALLOWABLE UNIFORM FLOOR LOAD (PLF)

Joist Span (ft)	PWI 90																
	9½"		11¾"		14"		16"		18"		20"		22"		24"		
	Live L/480	Total 100%															
6	-	467	-	467	-	467	-	467	-	467	-	467	-	800	-	800	
7	-	400	-	400	-	400	-	400	-	400	-	400	-	686	-	686	
8	-	350	-	350	-	350	-	350	-	350	-	350	-	600	-	600	
9	-	311	-	311	-	311	-	311	-	311	-	311	-	533	-	533	
10	-	280	-	280	-	280	-	280	-	280	-	280	-	480	-	480	
11	255	255	-	255	-	255	-	255	-	255	-	255	-	436	-	436	
12	203	233	-	233	-	233	-	233	-	233	-	233	-	400	-	400	
13	165	215	-	215	-	215	-	215	-	215	-	215	-	369	-	369	
14	135	200	-	200	-	200	-	200	-	200	-	200	-	343	-	343	
15	112	187	180	187	-	187	-	187	-	187	-	187	-	320	-	320	
16	94	175	152	175	-	175	-	175	-	175	-	175	-	300	-	300	
17		129	165	-	165	-	165	-	165	-	165	-	165	-	282	-	282
18		110	156	-	156	-	156	-	156	-	156	-	156	-	267	-	267
19		95	147	135	147	-	147	-	147	-	147	-	147	-	253	-	253
20		82	140	117	140	-	140	-	140	-	140	-	140	-	240	-	240
21				102	133	-	133	-	133	-	133	-	133	-	229	-	229
22				90	127	119	127	-	127	-	127	-	127	-	218	-	218
23				79	122	106	122	-	122	-	122	204	209	-	209		
24				71	117	94	117	-	117	-	117	182	200	-	200		
25						84	112	108	112	-	112	163	192	-	192		
26						75	108	96	108	-	108	147	185	175	185		
27						67	104	87	104	-	104	132	178	158	178		
28						61	100	78	100	98	100	119	171	143	171		
29								71	97	89	97	108	166	130	166		
30								64	93	81	93	99	160	118	160		
31								59	90	74	90	90	155	108	155		
32										67	88	82	150	99	150		
33										62	85	75	141	91	145		
34										57	82	69	133	83	141		
35										52	80	64	126	77	137		
36												59	118	71	129		
37												54	109	66	122		
38												50	101	61	116		
39														56	110		
40														52	105		
41														49	98		
42														46	91		

Notes:

1. Table values apply to uniformly loaded floor joists.
2. Span is measured to the center of each support.
3. The values in the Total columns are based on an L/240 total load deflection limit. Building codes typically require L/360 for live load. Experience has shown that a live load deflection limit of L/480 at 40 psf for residential floors does a better job than L/360 of meeting most performance expectations.
4. Table values do not account for stiffness added by glued or nailed sheathing.
5. Provide at least 1¼" of bearing length at end supports and 3½" at intermediate supports.
6. Provide lateral restraint at supports (e.g. blocking panels, rim board) and along the compression flange of each joist (e.g. floor sheathing, gypsum board ceiling).
7. Use sizing software or consult a professional engineer to analyze conditions outside the scope of this table (e.g. different bearing lengths, concentrated loads) or for multiple span joists if the length of any span is less than half the length of an adjacent span.

How to Use Floor Load Tables

1. Choose a joist spacing and convert the live and total design loads specified in pounds per square foot (psf) to joist loads in pounds per lineal foot (plf).
Joist Spacing [ft] x Design Load [psf] = Joist Load [plf]

JOIST LOAD (PLF)

Joist Spacing		Design Load (plf)									
Inches	Feet	20	30	40	50	60	70	80	90	100	
12	1	20	30	40	50	60	70	80	90	100	
16	1.33	27	40	53	67	80	93	106	120	133	
19.2	1.6	32	48	64	80	96	112	128	144	160	
24	2	40	60	80	100	120	140	160	180	200	

2. Choose a span and scan across the Span row to find a joist size with sufficient Live and Total load capacities. Both requirements must be satisfied. When no value is shown in a Live column, Total load governs.
3. Web stiffeners are required at all supports for 22" and 24" joists. See *Web Stiffener Requirements* on page 11 for more details.

Web Stiffener Requirements

Web stiffeners are pairs of small blocks, cut from panels or 2x4s, that are nailed to the joist web to stiffen a deep web, increase reaction capacity or accommodate a special connector. Web stiffeners are not required when joists are sized by means of the tables in this guide, with the following exceptions:

1. Web stiffeners are required at the ends of joists set in hangers that are not deep enough to laterally support the top flanges of the joists. Refer to the hanger manufacturer's installation instructions.
2. Web stiffeners are required to accommodate special connector nailing requirements. Refer to the connector manufacturer's installation instructions.
3. Web stiffeners are required at birdsmouth cuts at the low end supports of sloped joists.
4. Web stiffeners are required at all supports on 22- and 24-inch joists.

When joists are sized by means of sizing software, or otherwise engineered for an application, web stiffeners are required as follows:

1. Web stiffeners are required for high reactions at supports. Refer to an evaluation report.
2. Web stiffeners are required under concentrated loads applied to the tops of joists between supports, or along cantilevers beyond the support, when the concentrated load exceeds 1500 pounds.

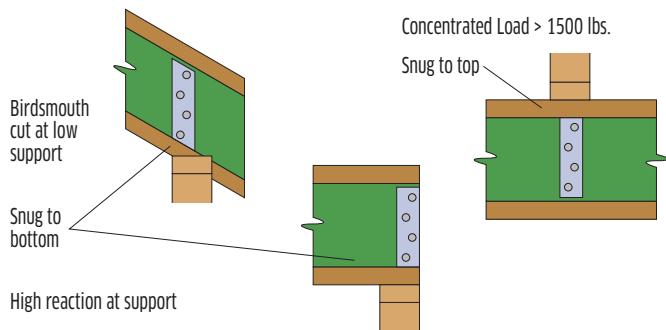
NUMBER OF WEB STIFFENER NAILS REQUIRED

Joist Depth	24" & 20"	18" & 16"	14" & Less
All Other Conditions	10	6	4

WEB STIFFENER SIZE REQUIRED

Flange Width	Minimum Dimensions	
	Thickness	Width
1½"	15/32"	2 5/16"
1¾"	19/32"	2 5/16"
2 1/8"	23/32"	2 5/16"
2 5/8"	23/32"	2 5/16"
2 1/2"	23/32"	2 5/16"
3 1/8"	1 1/2"	3 1/2"

Web stiffener length is approximately $\frac{1}{8}$ " less than the clear distance between flanges.



Web Hole Specifications

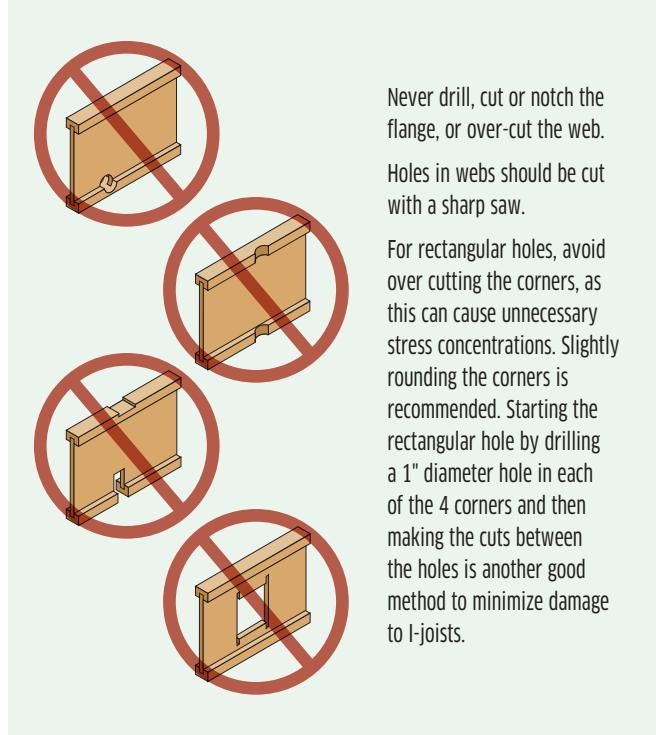
DUCT HOLES

Minimum Distance 'D' From Any Support to the Centerline of the Hole

Joist Series	Joist Span (ft)	Duct Hole Width								
		8"	10"	12"	14"	16"	18"	20"	22"	24"
20	≤ 8'	3'-10"	3'-11"	3'-11"						
	≤ 12'	5'-9"	5'-10"	5'-11"						
	≤ 16'	7'-8"	7'-10"	7'-11"						
	≤ 20'	9'-7"	9'-9"	9'-11"						
45	≤ 8'	3'-5"	3'-7"	3'-8"	3'-9"	3'-10"	4'-0"	(2)		
	≤ 12'	5'-2"	5'-4"	5'-6"	5'-8"	5'-10"	6'-0"	(2)		
	≤ 16'	6'-11"	7'-2"	7'-5"	7'-7"	7'-9"	8'-0"	(2)		
	≤ 20'	8'-8"	9'-0"	9'-3"	9'-6"	9'-9"	10'-0"	(2)		
60	≤ 8'	3'-6"	3'-7"	3'-9"	3'-10"	3'-11"	(2)	(2)		
	≤ 12'	5'-3"	5'-5"	5'-7"	5'-9"	5'-11"	(2)	(2)		
	≤ 16'	7'-0"	7'-3"	7'-6"	7'-8"	7'-10"	(2)	(2)		
	≤ 20'	8'-10"	9'-1"	9'-4"	9'-7"	9'-10"	(2)	(2)		
70	≤ 8'	3'-7"	3'-8"	3'-9"	3'-10"	(2)	(2)	(2)		
	≤ 12'	5'-5"	5'-6"	5'-8"	5'-10"	(2)	(2)	(2)		
	≤ 16'	7'-2"	7'-5"	7'-7"	7'-9"	(2)	(2)	(2)		
	≤ 20'	9'-0"	9'-3"	9'-6"	9'-9"	(2)	(2)	(2)		
77 depths to 20"(3)	≤ 8'	3'-8"	3'-9"	3'-11"	3'-11"	(2)	(2)	(2)		
	≤ 12'	5'-7"	5'-8"	5'-10"	5'-11"	(2)	(2)	(2)		
	≤ 16'	7'-5"	7'-7"	7'-10"	7'-11"	(2)	(2)	(2)		
	≤ 20'	9'-4"	9'-6"	9'-9"	9'-11"	(2)	(2)	(2)		
90 depths to 20"(3)	≤ 8'	11'-2"	11'-5"	11'-9"	11'-11"	(2)	(2)	(2)	(2)	
	≤ 12'	5'-7"	5'-8"	5'-10"	5'-11"	(2)	(2)	(2)	(2)	
	≤ 16'	7'-5"	7'-7"	7'-9"	7'-11"	(2)	(2)	(2)	(2)	
	≤ 20'	9'-4"	9'-6"	9'-8"	9'-11"	(2)	(2)	(2)	(2)	
	≤ 24'	11'-2"	11'-5"	11'-8"	11'-10"	(2)	(2)	(2)	(2)	

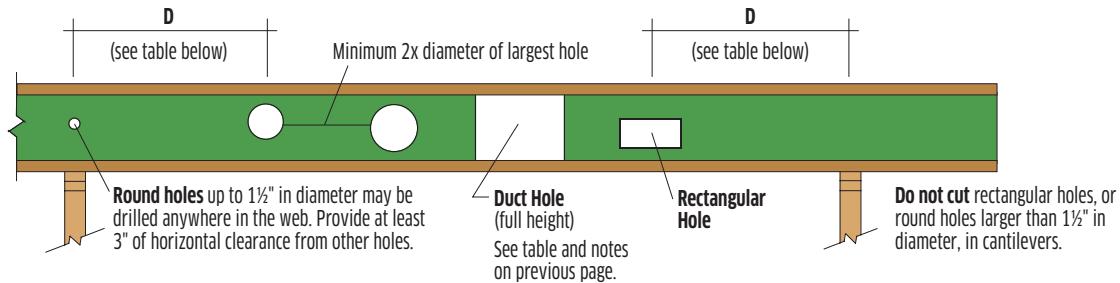
Notes:

- (1) For other joist spans, use sizing software to locate the duct hole
- (2) For this width, use sizing software to locate the duct hole
- (3) For joist depths greater than 20 inches, use sizing software to locate duct holes



To review Pacific Woodtech's Installation Guide, please visit www.pacificwoodtech.com/products.

Web Hole Specifications



ROUND AND RECTANGULAR HOLES

Minimum Distance 'D' From Any Support to the Centerline of the Hole

Round Hole Diameter		2"	3"	4"	5"	6"	6 1/4"	8 5/8"	10"	10 1/4"	12"	12 3/4"	14 1/4"	16 1/4"
Rectangular Hole Longest Side	Span (ft)	1 1/2"	2 1/4"	3"	3 3/4"	4 1/2"	4 5/8"	6 1/4"	7 1/2"	8"	9"	9 1/2"	11"	12 1/2"
9 1/2" Joist	8'	1'-1"	1'-7"	2'-1"	2'-8"	3'-2"	3'-4"							
	12'	1'-7"	2'-4"	3'-2"	3'-11"	4'-9"	5'-0"							
	16'	2'-1"	3'-2"	4'-3"	5'-3"	6'-4"	6'-8"							
11 1/8" Joist	8'	1'-1"	1'-2"	1'-2"	1'-8"	2'-2"	2'-3"	3'-6"						
	12'	1'-1"	1'-2"	1'-10"	2'-6"	3'-3"	3'-5"	5'-3"						
	16'	1'-1"	1'-5"	2'-5"	3'-4"	4'-4"	4'-7"	7'-0"						
	20'	1'-1"	1'-9"	3'-0"	4'-2"	5'-5"	5'-8"	8'-10"						
14" Joist	12'	1'-1"	1'-2"	1'-2"	1'-5"	2'-1"	2'-3"	3'-10"	4'-10"	5'-5"				
	16'	1'-1"	1'-2"	1'-2"	1'-10"	2'-9"	3'-0"	5'-2"	6'-5"	7'-3"				
	20'	1'-1"	1'-2"	1'-2"	2'-4"	3'-5"	3'-9"	6'-5"	8'-0"	9'-1"				
	24'	1'-1"	1'-2"	1'-5"	2'-9"	4'-2"	4'-6"	7'-8"	9'-7"	10'-11"				
16" Joist	16'	1'-1"	1'-2"	1'-2"	1'-3"	1'-4"	1'-6"	3'-7"	4'-9"	5'-5"	6'-7"	7'-5"		
	20'	1'-1"	1'-2"	1'-2"	1'-3"	1'-8"	1'-11"	4'-6"	6'-0"	6'-10"	8'-3"	9'-4"		
	24'	1'-1"	1'-2"	1'-2"	1'-3"	2'-0"	2'-4"	5'-5"	7'-2"	8'-2"	9'-11"	11'-2"		
	28'	1'-1"	1'-2"	1'-2"	1'-3"	2'-4"	2'-8"	6'-4"	8'-5"	9'-6"	11'-7"	13'-0"		
18" Joist	16'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	2'-2"	3'-3"	3'-11"	5'-0"	5'-7"	7'-7"	
	20'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	2'-8"	4'-1"	4'-11"	6'-2"	7'-0"	9'-6"	
	24'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	3'-2"	4'-11"	5'-10"	7'-5"	8'-5"	11'-5"	
	28'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	3'-9"	5'-9"	6'-10"	8'-8"	9'-9"	13'-4"	
20" Joist	16'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-4"	1'-10"	2'-5"	3'-6"	4'-1"	5'-9"	7'-9"
	20'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-4"	2'-3"	3'-1"	4'-4"	5'-1"	7'-2"	9'-9"
	24'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-4"	2'-9"	3'-8"	5'-2"	6'-1"	8'-7"	11'-8"
	28'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-4"	3'-2"	4'-3"	6'-1"	7'-2"	10'-0"	13'-7"
22" Joist	16'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	2'-7"	3'-4"	3'-9"	4'-5"	4'-10"	6'-0"	7'-1"
	20'	1'-1"	1'-2"	1'-2"	1'-3"	1'-5"	1'-7"	3'-2"	4'-2"	4'-8"	5'-7"	6'-1"	7'-6"	8'-10"
	24'	1'-1"	1'-2"	1'-2"	1'-3"	1'-8"	1'-10"	3'-10"	5'-0"	5'-7"	6'-8"	7'-3"	8'-11"	10'-7"
	28'	1'-1"	1'-2"	1'-2"	1'-3"	1'-11"	2'-2"	4'-6"	5'-10"	6'-7"	7'-9"	8'-6"	10'-5"	12'-5"
24" Joist	16'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-10"	2'-7"	3'-0"	3'-8"	4'-0"	5'-1"	6'-2"
	20'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	2'-3"	3'-2"	3'-8"	4'-6"	5'-0"	6'-4"	7'-8"
	24'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	2'-9"	3'-10"	4'-5"	5'-5"	6'-0"	7'-8"	9'-3"
	28'	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	3'-2"	4'-6"	5'-2"	6'-4"	7'-0"	8'-11"	10'-9"

See General Notes below.

General Notes

- Table values apply to joists sized by means of the load or span tables in this publication. Use beam sizing software for a more precise analysis or to analyze conditions outside of the scope of these tables.
- Web holes may be located anywhere between the joist flanges. Leave at least $\frac{1}{8}$ inch clearance between the edges of holes and the flanges.
- Do not cut rectangular holes, or round holes larger than 1 1/2 inch diameter, in cantilevers.
- The horizontal clearance between the edges of adjacent holes must be at least twice the diameter (or longest side) of the larger hole. Exception: A 1 1/2 inch diameter hole may be drilled anywhere in the web. Provide at least 3 inches of horizontal clearance from adjacent holes of any size.

2.0E PWLVL

A better alternative than
traditional sawn lumber pieces.

1 1/4" PWLVL REFERENCE DESIGN VALUES

Depth (in)	1.5E PWLVL						2.0E PWLVL							
	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)
	100%	115%	125%	100%	115%	125%		100%	115%	125%	100%	115%	125%	
3 1/2	939	1080	1174	857	986	1071	9	1164	1338	1455	1181	1358	1476	13
5 1/2	1476	1697	1845	1934	2224	2417	36	1829	2103	2286	2664	3064	3330	49
7 1/4	1945	2237	2432	3179	3656	3974	83	2411	2772	3013	4380	5037	5475	111
9 1/4	2482	2854	3103	4929	5669	6162	173	3076	3537	3845	6791	7810	8489	231
9 1/2	2549	2932	3186	5172	5947	6465	188	3159	3633	3948	7125	8194	8907	250
11 1/4	3019	3472	3773	7011	8063	8764	311	3741	4302	4676	9660	11109	12075	415
11 1/8	3186	3664	3983	7728	8887	9660	366	3948	4541	4936	10647	12245	13309	488
14	3757	4320	4696	10393	11952	12992	600	4655	5353	5819	14320	16468	17900	800
16	4293	4937	5367	13217	15200	16522	896	5320	6118	6650	18210	20942	22763	1195
18	4830	5555	6038	16339	18789	20423	1276	5985	6883	7481	22511	25888	28139	1701
20	5367	6172	6708	19751	22713	24688	1750	6650	7648	8313	27212	31294	34015	2333
22	5903	6789	7379	23447	26964	29309	2329	7315	8412	9144	32035	37150	40381	3106
24	6406	7367	8008	27166	31241	33957	2977	7938	9129	9923	37428	43043	46786	3969

AVAILABLE SIZES (INCHES)

1.5E AND 2.0E PWLVL		2.0E AND 2.2E PWLVL		
1 1/2"	1 3/4"	3 1/2"	5 1/4"	7"
3 1/2	3 1/2	-	-	-
5 1/2	5 1/2	5 1/2	5 1/2	-
7 1/4	7 1/4	7 1/4	7 1/4	7 1/4
9 1/4	9 1/4	9 1/4	9 1/4	9 1/4
9 1/2	9 1/2	9 1/2	9 1/2	9 1/2
11 1/4	11 1/4	11 1/4	11 1/4	11 1/4
11 1/8	11 1/8	11 1/8	11 1/8	11 1/8
14	14	14	14	14
16	16	16	16	16
18	18	18	18	18
20	20	20	20	20
22	22	22	22	22
24	24	24	24	24

3 1/2" PWLVL REFERENCE DESIGN VALUES

Depth (in)	2.0E PWLVL						2.2E PWLVL—AVAILABLE UPON REQUEST							
	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)
	100%	115%	125%	100%	115%	125%		100%	115%	125%	100%	115%	125%	
3 1/2	2328	2677	2909	2362	2716	2952	25	2328	2677	2909	2362	2716	2952	28
5 1/2	3658	4206	4572	5328	6128	6660	97	3658	4206	4572	5328	6128	6660	107
7 1/4	4821	5544	6027	8761	10075	10951	222	4821	5544	6027	8761	10075	10951	245
9 1/4	6151	7074	7689	13583	15620	16978	462	6151	7074	7689	13583	15620	16978	508
9 1/2	6318	7265	7897	14251	16388	17813	500	6318	7265	7897	14251	16388	17813	550
11 1/4	7481	8603	9352	19320	22118	24150	831	7481	8603	9352	19320	22118	24150	914
11 1/8	7897	9081	9871	21295	24489	26619	977	7897	9081	9871	21295	24489	26619	1075
14	9310	10707	11638	28639	32935	35799	1601	9310	10707	11638	28639	32935	35799	1761
16	10640	12236	13300	36421	41884	45526	2389	10640	12236	13300	36421	41884	45526	2628
18	11970	13766	14963	45022	51775	56277	3402	11970	13766	14963	45022	51775	56277	3742
20	13300	15295	16625	54424	62587	68030	4667	13300	15295	16625	54424	62587	68030	5133
22	14630	16825	18288	64609	74301	80761	6211	14630	16825	18288	64609	74301	80761	6832
24	15877	18258	19846	74857	86085	93571	7939	15877	18258	19846	74857	86085	93571	8733

EQUIVALENT SPECIFIC GRAVITY FOR FASTENER DESIGN

Nails & Wood Screws	Face	Lateral	0.50
	Withdrawal	0.50	
Bolts & Lag Screws	Edge	Lateral	0.50
	Withdrawal	0.47	
Bolts & Lag Screws	Face	Lateral	0.50
	Edge	Lateral	NA

1.5E PWLVL REFERENCE DESIGN VALUES⁽¹⁾

Modulus of Elasticity E =	1500000 psi ⁽²⁾
Bending (beam) F _b =	2250 psi ⁽³⁾⁽⁴⁾
Horizontal Shear (beam) F _v =	230 psi
Compression Perpendicular to Grain (beam) F _{cL} =	750 psi ⁽²⁾

2.0E PWLVL REFERENCE DESIGN VALUES⁽¹⁾

Modulus of Elasticity E =	2000000 psi ⁽²⁾
Bending (beam) F _b =	3100 psi ⁽³⁾⁽⁴⁾
Horizontal Shear (beam) F _v =	285 psi
Compression Perpendicular to Grain (beam) F _{cL} =	850 psi ⁽²⁾

2.2E PWLVL REFERENCE DESIGN VALUES⁽¹⁾

Modulus of Elasticity E =	2200000 psi ⁽²⁾
Bending (beam) F _b =	3100 psi ⁽³⁾⁽⁴⁾
Horizontal Shear (beam) F _v =	285 psi
Compression Perpendicular to Grain (beam) F _{cL} =	850 psi ⁽²⁾

(1) Values apply to dry service conditions

(2) Do not adjust for load duration

(3) Adjust by $(12/d)^{1/5}$, where d is the depth of the member [inches]

(4) Adjust by 1.04 for repetitive members as defined in the ANSI/AWC NDS

7" PWLVL REFERENCE DESIGN VALUES

Depth (in)	2.0E PWLVL						2.2E PWLVL—AVAILABLE UPON REQUEST							
	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)	Maximum Vertical Shear (lb)			Maximum Bending Moment (ft-lb)			EI (x 10 ⁶ lb-in ²)
	100%	115%	125%	100%	115%	125%		100%	115%	125%	100%	115%	125%	
7 1/4	9643	11089	12053	17522	20150	21902	445	9643	11089	12053	17522	20150	21902	489
9 1/4	12303	14148	15378	27166	31240	33957	923	12303	14148	15378	27166	31240	33957	1016
9 1/2	12635	14530	15794	28501	32777	35627	1000	12635	14530	15794	28501	32777	35627	1100
11 1/4	14963	17207	18703	38640	44436	48300	1661	14963	17207	18703	38640	44436	48300	1827
11 1/8	15794	18163	19742	42590	48978	53237	1954	15794	18163	19742	42590	48978	53237	2149
14	18620	21413	23275	57279	65871	71599	3201	18620	21413	23275	57279	65871	71599	3521
16	21280	24472	26600	72842	83768	91052	4779	21280	24472	26600	72842	83768	91052	5257
18	23940	27531	29925	90044	103550	112555	6804	23940	27531	29925	90044</			

One-Ply 1¾" 2.0E Beam

ALLOWABLE UNIFORM LOADS*—POUNDS PER LINEAL FOOT

ONE-PLY x 1¾" 2.0E PWLVL

Span (ft)	Key	3½"	5½"	7¼"	9¼"	9½"	11¼"	11¾"	14"
6	LL	86	333	762	-	-	-	-	-
	TL	127	497	763	1028	1063	1325	1425	1796
	BRG	1.5/3	1.5/3	1.5/3.9	2.1/5.2	2.2/5.4	2.7/6.7	2.9/7.2	3.6/9.1
7	LL	54	210	480	-	-	-	-	-
	TL	71	278	636	849	877	1083	1161	1445
	BRG	1.5/3	1.5/3	1.5/3.8	2/5	2.1/5.2	2.6/6.4	2.7/6.9	3.4/8.5
8	LL	140	322	668	724	-	-	-	-
	TL	162	374	723	746	916	979	1208	-
	BRG	1.5/3	1.5/3	1.5/3	2/4.9	2/5	2.5/6.2	2.6/6.6	3.3/8.2
9	LL	99	226	469	508	-	-	-	-
	TL	100	232	629	649	793	846	1038	-
	BRG	1.5/3	1.5/3	1.5/3	1.9/4.8	2/4.9	2.4/6	2.6/6.4	3.2/7.9
10	LL	-	-	342	370	615	724	-	-
	TL	65	151	509	551	699	745	909	-
	BRG	1.5/3	1.5/3	1.5/3	1.7/4.3	1.9/4.7	2.4/5.9	2.5/6.3	3.1/7.7
11	LL	-	-	257	278	462	544	-	-
	TL	44	102	381	413	625	665	809	-
	BRG	1.5/3	1.5/3	1.5/3	1.5/3.6	1.5/3.9	2.3/5.8	2.5/6.2	3/7.5
12	LL	-	-	198	214	356	419	686	-
	TL	-	-	71	293	317	529	586	729
	BRG	-	-	1.5/3	1.5/3	2.2/5.4	2.4/6	3/7.4	-
13	LL	-	-	156	169	280	329	540	-
	TL	-	-	51	229	249	415	489	663
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.8/4.6	2.2/5.4	2.9/7.3
14	LL	-	-	125	135	224	264	432	-
	TL	-	-	37	183	198	331	390	578
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.6/4	1.9/4.7	2.8/6.9
15	LL	-	-	101	110	182	214	351	-
	TL	-	-	148	160	268	316	503	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.4	1.6/4.1	2.6/6.4	-
16	LL	-	-	83	90	150	177	289	-
	TL	-	-	121	131	220	260	428	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.6	2.3/5.8	-
17	LL	-	-	70	75	125	147	241	-
	TL	-	-	100	109	183	216	356	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.2	2.1/5.2	-
18	LL	-	-	59	64	105	124	203	-
	TL	-	-	84	91	153	181	299	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3	1.8/4.6	-
19	LL	-	-	54	90	105	173	-	-
	TL	-	-	77	129	153	253	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3	1.7/4.1	-
20	LL	-	-	77	90	148	-	-	-
	TL	-	-	110	130	216	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.7	-	-
21	LL	-	-	66	78	128	-	-	-
	TL	-	-	95	112	186	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.4	-	-
22	LL	-	-	58	68	111	-	-	-
	TL	-	-	82	97	161	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.1	-	-
23	LL	-	-	59	97	-	-	-	-
	TL	-	-	-	-	84	140	-	-
	BRG	-	-	-	-	1.5/3	1.5/3	-	-
24	LL	-	-	-	-	-	86	-	-
	TL	-	-	-	-	-	122	-	-
	BRG	-	-	-	-	-	1.5/3	-	-
25	LL	-	-	-	-	-	76	-	-
	TL	-	-	-	-	-	107	-	-
	BRG	-	-	-	-	-	1.5/3	-	-
26	LL	-	-	-	-	-	67	-	-
	TL	-	-	-	-	-	95	-	-
	BRG	-	-	-	-	-	1.5/3	-	-
27	LL	-	-	-	-	-	60	-	-
	TL	-	-	-	-	-	84	-	-
	BRG	-	-	-	-	-	1.5/3	-	-
28	LL	-	-	-	-	-	54	-	-
	TL	-	-	-	-	-	75	-	-
	BRG	-	-	-	-	-	1.5/3	-	-
29	LL	-	-	-	-	-	-	-	-
	TL	-	-	-	-	-	-	-	-
	BRG	-	-	-	-	-	-	-	-
30	LL	-	-	-	-	-	-	-	-
	TL	-	-	-	-	-	-	-	-
	BRG	-	-	-	-	-	-	-	-

* Can be applied to the beam in addition to its own weight.

Simple or multiple beam spans.

2 plies minimum for depths greater than 14 inches.

Wax-based sealer applied to mitigate moisture issues associated with wood products during storage and construction.

Key to Table:

LL = Maximum live load - limits deflection to L/360

TL = Maximum total load - limits deflections to L/240 (or a maximum of 0.3125" for beams 7¼" deep or less)

BRG = Required end/intermediate bearing length (inches), based on bearing stress of 850 psi.

Two-Ply 1¾" 2.0E Beam

ALLOWABLE UNIFORM LOADS*-POUNDS PER LINEAL FOOT

TWO-PLY X 1¾" 2.0E PWLVL

Span (ft)	Key	3½"	5½"	7¼"	9¼"	9½"	11¼"	11½"	14"	16"	18"	20"	22"	24"
6	LL	172	666	1525	-	-	-	-	-	-	-	-	-	-
	TL	254	993	1526	2056	2127	2650	2850	3591	4388	5304	6366	7613	8997
	BRG	1.5/3	1.5/3	1.5/3.9	2.1/5.2	2.2/5.4	2.7/6.7	2.9/7.2	3.6/9.1	4.4/11.1	5.4/13.4	6.4/16.1	7.7/19.2	9.1/22.7
7	LL	108	419	960	-	-	-	-	-	-	-	-	-	-
	TL	141	556	1272	1698	1754	2166	2322	2889	3484	4147	4893	5736	6634
	BRG	1.5/3	1.5/3	1.5/3.8	2/5	2.1/5.2	2.6/6.4	2.7/6.9	3.4/8.5	4.1/10.3	4.9/12.2	5.8/14.4	6.8/16.9	7.8/19.6
8	LL	281	643	1336	1447	-	-	-	-	-	-	-	-	-
	TL	324	747	1446	1493	1831	1958	2416	2887	3404	3972	4600	5252	-
	BRG	1.5/3	1.5/3	2/4.9	2/5	2.5/6.2	2.6/6.6	3.3/8.2	3.9/9.8	4.6/11.5	5.4/13.4	6.2/15.5	7.1/17.7	-
9	LL	197	452	938	1016	-	-	-	-	-	-	-	-	-
	TL	200	464	1259	1298	1586	1693	2075	2465	2885	3342	3838	4346	-
	BRG	1.5/3	1.5/3	1.9/4.8	2/4.9	2.4/6	2.6/6.4	3.2/7.9	3.8/9.4	4.4/11	5.1/12.7	5.8/14.6	6.6/16.5	-
10	LL	-	-	684	741	1230	1447	-	-	-	-	-	-	-
	TL	130	302	1018	1103	1398	1490	1819	2150	2504	2884	3292	3705	-
	BRG	1.5/3	1.5/3	1.7/4.3	1.9/4.7	2.4/5.9	2.5/6.3	3.1/7.7	3.6/9.1	4.2/10.6	4.9/12.2	5.6/13.9	6.3/15.7	-
11	LL	-	-	514	557	924	1087	-	-	-	-	-	-	-
	TL	87	204	762	826	1250	1331	1618	1905	2211	2535	2882	3228	-
	BRG	1.5/3	1.5/3	1.5/3.6	1.5/3.9	2.3/5.8	2.5/6.2	3/7.5	3.5/8.9	4.1/10.3	4.7/11.8	5.4/13.4	6/15	-
12	LL	-	-	396	429	712	837	1372	-	-	-	-	-	-
	TL	-	-	142	585	635	1058	1172	1457	1711	1979	2262	2562	2860
	BRG	-	-	1.5/3	1.5/3	1.5/3.2	2.2/5.4	2.4/6	3/7.4	3.5/8.7	4/10.1	4.6/11.5	5.2/13	5.8/14.5
13	LL	-	-	311	337	560	659	1079	-	-	-	-	-	-
	TL	-	-	102	459	497	830	977	1325	1552	1790	2041	2305	2566
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.8/4.6	2.2/5.4	2.9/7.3	3.4/8.6	3.9/9.9	4.5/11.2	5.1/12.7	5.7/14.1
14	LL	-	-	249	270	448	527	864	1290	-	-	-	-	-
	TL	-	-	74	365	396	662	780	1156	1420	1635	1859	2095	2327
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.6/4	1.9/4.7	2.8/6.9	3.4/8.4	3.9/9.7	4.4/11	5/12.4	5.5/13.8
15	LL	-	-	203	220	365	429	703	1049	1493	-	-	-	-
	TL	-	-	296	321	537	632	1006	1280	1504	1707	1920	2128	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.4	1.6/4.1	2.6/6.4	3.3/8.2	3.8/9.6	4.3/10.9	4.9/12.2	5.4/13.5	-
16	LL	-	-	167	181	300	353	579	864	1230	-	-	-	-
	TL	-	-	242	263	440	519	856	1124	1391	1578	1771	1960	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.6	2.3/5.8	3.1/7.7	3.8/9.5	4.3/10.7	4.8/12	5.3/13.3	-
17	LL	-	-	139	151	250	295	483	720	1026	1407	-	-	-
	TL	-	-	200	218	365	431	711	994	1230	1466	1644	1817	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.2	2.1/5.2	2.9/7.2	3.6/8.9	4.2/10.6	4.8/11.9	5.3/13.1	-
18	LL	-	-	117	127	211	248	407	607	864	1185	-	-	-
	TL	-	-	168	182	306	361	597	885	1095	1326	1534	1693	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3	1.8/4.6	2.7/6.8	3.4/8.4	4.1/10.2	4.7/11.8	5.2/13	-
19	LL	-	-	108	179	211	346	516	735	1008	1342	-	-	-
	TL	-	-	153	259	306	506	760	981	1188	1412	1584	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.7/4.1	2.5/6.2	3.2/8	3.9/9.6	4.6/11.4	5.1/12.8	-	-
20	LL	-	-	154	181	296	442	630	864	1150	1470	-	-	-
	TL	-	-	220	261	432	649	884	1070	1272	1475	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.7	2.2/5.6	3/7.6	3.7/9.1	4.3/10.9	5/12.6	-	-
21	LL	-	-	133	156	256	382	544	747	994	1152	1336	-	-
	TL	-	-	189	224	371	559	800	969	1152	1336	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.4	2/5.1	2.9/7.2	3.5/8.7	4.1/10.3	4.8/12	-	-
22	LL	-	-	116	136	223	332	473	649	864	1105	-	-	-
	TL	-	-	163	193	321	484	694	881	1048	1216	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.5/3.1	1.8/4.6	2.6/6.6	3.3/8.3	3.9/9.9	4.6/11.4	-	-
23	LL	-	-	119	195	291	414	568	756	967	-	-	-	-
	TL	-	-	168	280	422	605	805	957	1110	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	1.7/4.2	2.4/6	3.2/8	3.8/9.4	4.4/10.9	-	-	-
24	LL	-	-	172	256	365	500	666	851	-	-	-	-	-
	TL	-	-	245	370	530	732	877	1018	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3	2.2/5.5	3/7.6	3.6/9	4.2/10.5	-	-	-	-
25	LL	-	-	152	227	323	442	589	753	-	-	-	-	-
	TL	-	-	215	325	467	646	807	936	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.6	2/5.1	2.8/7	3.5/8.7	4/10.1	-	-	-	-
26	LL	-	-	135	201	287	393	524	669	-	-	-	-	-
	TL	-	-	190	288	414	572	745	864	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.3	1.9/4.7	2.6/6.4	3.3/8.4	3.9/9.7	-	-	-	-
27	LL	-	-	120	180	256	351	468	598	-	-	-	-	-
	TL	-	-	168	255	368	509	681	800	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.1	1.7/4.4	2.4/6	3.2/8	3.7/9.3	-	-	-	-
28	LL	-	-	108	161	230	315	419	536	-	-	-	-	-
	TL	-	-	149	227	328	454	609	742	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.4	1.6/4.1	2.2/5.6	3/7.4	3.6/9	-	-	-	-
29	LL	-	-	145	207	283	377	482	-	-	-	-	-	-
	TL	-	-	203	294	407	546	690	-	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.8	2.1/5.2	2.8/6.9	3.5/8.7	-	-	-	-	-
30	LL	-	-	131	187	256	341	436	632	-	-	-	-	-
	TL	-	-	182	264	366	491	-	-	-	-	-	-	-
	BRG	-	-	1.5/3	1.5/3	1.5/3.5	1.9/4.8	2.6/6.4	3.3/8.2	-	-	-	-	-

* Can be applied to the beam in addition to its own weight.

Simple or multiple beam spans.

2 plies minimum for depths greater than 14 inches.

Wax-based sealer applied to mitigate moisture issues associated with wood products during storage and construction.

Key to Table:

LL = Maximum live load - limits deflection to L/360

TL = Maximum total load - limits deflections to L/240 (or a maximum of 0.3125" for beams 7¼" deep or less)

BRG = Required end/intermediate bearing length (inches), based on bearing stress of 850 psi.

1.5E PWLVL Rim Board

1.5E PWLVL RIM BOARD REFERENCE DESIGN VALUES⁽¹⁾

Horizontal Load	=	200 plf ⁽²⁾ Fasten to the wall plate with 8d box or common nails at 6" o.c. Value applies to a ten minute wind or earthquake load duration ($C_D = 1.60$)
Vertical Load	=	3450 plf ⁽²⁾
½" Diameter Lag Screw or Bolt Lateral Load	=	350 lb ⁽³⁾

1.5E PWLVL REFERENCE DESIGN VALUES⁽¹⁾

Modulus of Elasticity E	=	1,500,000 psi ⁽²⁾
Bending (beam) F_b	=	2,250 psi ⁽³⁾ May be adjusted by $(12/d)^{1/5}$, where d is the depth of the member (inches)
		May be adjusted by 104 for repetitive members as defined in ANSI/AIF&PA NDS
Horizontal Shear (beam) F_v	=	230 psi ⁽³⁾
Compression Perpendicular to Grain (beam) $F_{c\perp}$	=	750 psi ⁽²⁾

Notes:

1. Values apply to dry service conditions
2. Do not adjust for load duration
3. May be adjusted for load duration

EQUIVALENT SPECIFIC GRAVITY FOR FASTENER DESIGN

Nails & Wood Screws	Face	Lateral	0.50
		Withdrawal	0.50
Bolts & Lag Screws	Edge	Lateral	0.50
		Withdrawal	0.47

CLOSEST ON-CENTER SPACING for a single row of nails in the narrow face

Nail Size	Spacing
8d common (2½" x 0.131")	3"
10d common (3" x 0.148")	4"
16d common (3½" x 0.162")	6" ⁽¹⁾

1. May be 4" when nailing through bottom wall plate and sheathing (maximum 1¾" penetration).

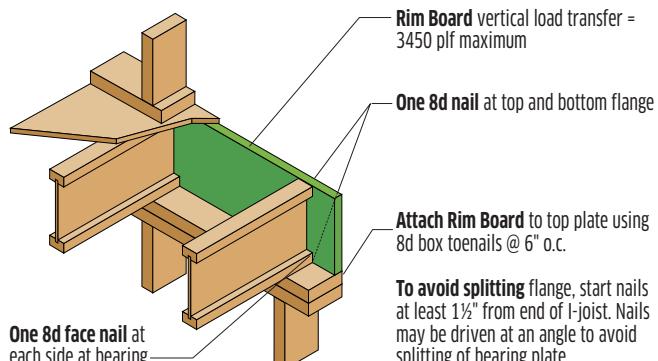
1¼", 1½", AND 1¾" 1.5E PWLVL RIM BOARD

AVAILABLE SIZES (INCHES):

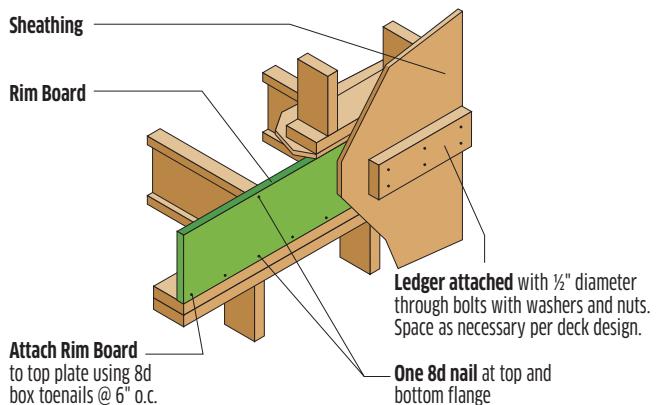
9½	11½	14	16

WEIGHTS (PLF):

3.1	3.9	4.5	5.2



DECK ATTACHMENT



2.0E PWLVL Columns

The properties that make PWLVL a superior beam material make it ideal for column use as well. In PWLVL columns, you'll find only quality construction, free of deep cracks, checks or twists. These columns are desirable enough to leave exposed, for a beautiful finish.

2.0E PWLVL COLUMNS ARE AVAILABLE IN:

3½" x 3½"	-	-
3½" x 5½"	5¾" x 5½"	-
3½" x 7½"	5¾" x 7½"	7 x 7½"

Reference Column Design Values

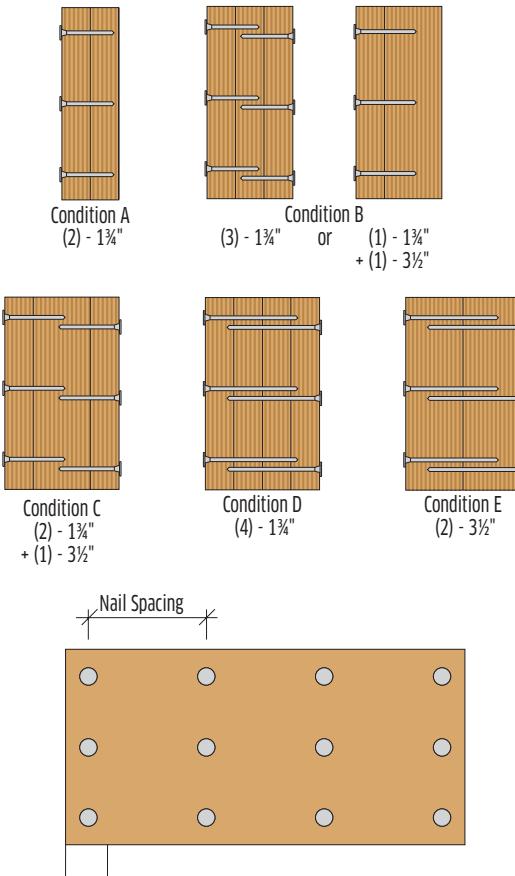
E	=	2,000,000 psi
$COVE$	=	0.10
$F_b\text{-BEAM}$	=	3100 psi x $(12/d_1)^{1/5}$
d_1	=	wide-face dimension [inches]
$F_b\text{-PLANK}$	=	3100 psi x $(1.75/d_2)^{1/3}$
d_2	=	narrow-face dimension [feet]
F_c	=	2750 psi

Contact us for special-order column sizes at www.pacificwoodtech.com/PWLVL or call 888.707.2285.

Multiple-Ply PWLVL Beam Assembly

COMBINATIONS OF 1 1/4" AND 3 1/2" PLIES

NAILS



1 1/4" AND 3 1/2" PLIES—MAXIMUM UNIFORM SIDE LOAD (PLF)

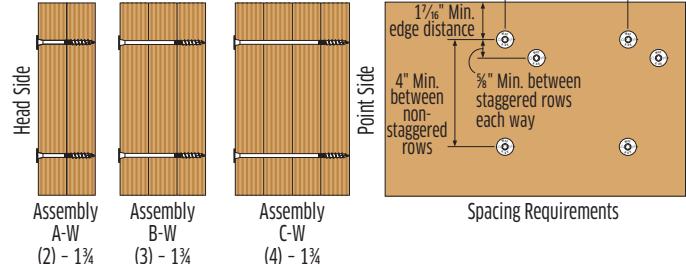
Condition	3 1/4" x 0.131" Nails		16d Common Nails	
	2 Rows at 12" o.c.	3 Rows at 12" o.c.	2 Rows at 12" o.c.	3 Rows at 12" o.c.
Condition A (2-1 1/4")	390	585	565	845
Condition B (3-1 1/4" OR 1-1 1/4" + 1-3 1/2")	290	435	425	635
Condition C (2-1 1/4" + 1-3 1/2")	260	390	375	565
Condition D (4-1 1/4")	Use bolts for this condition			
Condition E (2-3 1/2")	Use bolts for this condition			

Notes:

- Minimum fastener schedule for smaller side loads and top-loaded beams:
Conditions A, B & C, beams 12" deep or less: 2 rows 3 1/4" x 0.131" at 12" o.c.
Conditions A, B & C, beams deeper than 12": 3 rows 3 1/4" x 0.131" at 12" o.c.
Conditions D & E, all beam depths: 2 rows 1/2" bolts at 24" o.c.
- The table values for nails may be doubled for 6" o.c. and tripled for 4" o.c. nail spacings.
- The nail schedules shown apply to both sides of a three-ply beam.
- The table values apply to bolts meeting the requirements of ANSI/ASME Standard B18.2.1. A standard cut washer, or metal plate or strap of equal or greater dimensions, shall be provided between the wood and the bolt head and between the wood and the nut. The distance from the edge of the beam to the bolt holes must be at least 2" for 1/2" bolts. Bolt holes shall be the same diameter as the bolt.
- 7" wide beams must be loaded from both sides and/or top loaded.
- Beams wider than 7" must be designed by the engineer of record.
- Load duration factors may be applied to the table values.
- For proprietary fastener alternatives, consult the manufacturer's literature.

COMBINATIONS OF 1 1/4" PLIES

STRONG-DRIVE® SDW STRUCTURAL WOOD SCREWS



SIDELOADED 1 1/4" MULTI-PLY SCL ASSEMBLIES – ALLOWABLE UNIFORM LOAD APPLIED TO EITHER OUTSIDE MEMBER

Assembly	Components	Nominal Screw Length (in)	Loaded Side	Structural Composite Lumber					
				SDW @ 12" o.c.		SDW @ 16" o.c.		SDW @ 24" o.c.	
				2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows
A-W	2-ply SCL	3 1/8"	Either	1600	2400	1200	1800	800	1200
B-W	3-ply SCL	5	Head	1200	1800	900	1350	600	900
			Point	900	1350	675	1015	450	675
C-W	4-ply SCL	6 1/8"	Head	1065	1600	800	1200	535	800
			Point	800	1200	600	900	400	600

- Each ply is assumed to carry same proportion of load.
- Loads may be applied to the head side and point side concurrently provided neither published allowable load is exceeded. (Example: a 3-ply assembly with a head side load of 1300 plf and point side load of 1000 plf may be fastened together with 3 rows of SDW @ 16" o.c.)
- When hangers are installed on point side, hanger face fasteners must be a minimum of 3" long.
- Tables are based on Main Member Penetration as noted in Single-Fastener Load Tables of the Simpson Strong-Tie Fastening Systems 2014-2015 Catalog C-f-14 (page 302).

Installation

- SDW screws install best with a low-speed 1/2" drill and a T-40 6-lobe bit. The matched bit included with the screws is recommended for best results.
- Screw heads that are countersunk flush to the wood surface are acceptable if the screw has not spun out.
- Individual screw locations may be adjusted up to 3" to avoid conflicts with other hardware or to avoid lumber defects.

SCREW DIMENSIONS

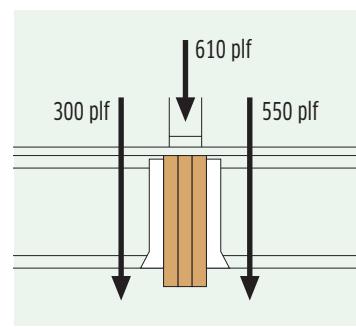
Model No.	Nominal Screw Length (L) (in)	Thread Length (TL) (in)	Head Stamp Length
SDW22338	3 1/8"	1 1/16"	3.37
SDW22500	5	1 1/16"	5.00
SDW22634	6 1/8"	1 1/16"	6.75

• Pre-drilling is typically not required.

How to Use the Maximum Uniform Side Load Table

EXAMPLE: THREE 1 1/4" PLIES LOADED FROM BOTH SIDES AND ABOVE (COND. B)

- Use allowable load tables or sizing software to size the beam to carry a total load of $(300 + 610 + 550) = 1460$ plf.
- Refer to the Condition B row in the table. Scan across the row from left to right for a table value greater than 550 plf, which is the greatest side load carried by the beam. The fourth value in the row indicates that 3 rows of 16d common nails at 12" o.c. will accommodate a side load of 635 plf which is greater than the 550 plf required. Use 3 rows of 16d common nails at 12" o.c., from both sides, to assemble the beam.





PWLVL Dimension

Laminated Veneer Lumber Engineered for Structural Framing

Extra-long PWLVL Dimension floor framing offers a stronger, stiffer, and straighter product than dimension lumber for all your structural applications. PWLVL Dimension is competitive in materials cost and is easy to handle and install, which can result in shorter construction schedules, saving you time and money. Build with confidence.

Use beam-calculating software for better optimization of material selection and on-center spacing.

PWLVL Dimension is available in virtually any length.

PWLVL DIMENSION DESIGN PROPERTY COMPARISON⁽¹⁾⁽²⁾

Product	Modulus of Elasticity E (psi)	Bending F _b (psi) ⁽³⁾	Horizontal Shear F _v (psi)	Compression Parallel to Grain F _c (psi) ⁽⁴⁾
2x4	1.5" x 3.5" x 2.0E PWLVL	2000000	4125	285
	1.5" x 3.5" x 1.8E PWLVL	1800000	3660	285
	1.5" x 3.5" x 1.5E PWLVL	1500000	2995	230
	2x4 Douglas Fir-Larch No. 2	1600000	1555	180
	2x4 Spruce-Pine-Fir No. 1 / No. 2	1400000	1510	135
	2x4 Hem-Fir No. 2	1300000	1465	150
	2x4 Western Woods No. 2	1000000	1165	135
2x6	1.5" x 5.5" x 2.0E PWLVL	2000000	3770	285
	1.5" x 5.5" x 1.8E PWLVL	1800000	3345	285
	1.5" x 5.5" x 1.5E PWLVL	1500000	2735	230
	2x6 Douglas Fir-Larch No. 2	1600000	1345	180
	2x6 Spruce-Pine-Fir No. 1 / No. 2	1400000	1310	135
	2x6 Hem-Fir No. 2	1300000	1270	150
	2x6 Western Woods No. 2	1000000	1010	135
2x8	1.5" x 7.25" x 2.0E PWLVL	2000000	3565	285
	1.5" x 7.25" x 1.8E PWLVL	1800000	3165	285
	1.5" x 7.25" x 1.5E PWLVL	1500000	2590	230
	2x8 Douglas Fir-Larch No. 2	1600000	1240	180
	2x8 Spruce-Pine-Fir No. 1 / No. 2	1400000	1205	135
	2x8 Hem-Fir No. 2	1300000	1175	150
	2x8 Western Woods No. 2	1000000	930	135
2x10	1.5" x 9.25" x 2.0E PWLVL	2000000	3395	285
	1.5" x 9.25" x 1.8E PWLVL	1800000	3015	285
	1.5" x 9.25" x 1.5E PWLVL	1500000	2465	230
	2x10 Douglas Fir-Larch No. 2	1600000	1140	180
	2x10 Spruce-Pine-Fir No. 1 / No. 2	1400000	1105	135
	2x10 Hem-Fir No. 2	1300000	1075	150
	2x10 Southern Pine No. 2	1400000	920	175
2x12	1.5" x 11.25" x 2.0E PWLVL	2000000	3265	285
	1.5" x 11.25" x 1.8E PWLVL	1800000	2895	285
	1.5" x 11.25" x 1.5E PWLVL	1500000	2370	230
	2x12 Douglas Fir-Larch No. 2	1600000	1035	180
	2x12 Spruce-Pine-Fir No. 1 / No. 2	1400000	1005	135
	2x12 Hem-Fir No. 2	1300000	975	150
	2x12 Southern Pine No. 2	1400000	860	175

(1) Refer to APA PR-L233 for PWLVL adjustment factors and other design properties.

(2) Refer to the 2015 NDS® for lumber adjustment factors and other design properties.

(3) Load applied to the narrow face of the member. Repetitive member and size factors have been applied where applicable.

(4) Size factors have been applied to lumber values where applicable.

For information about our complete line of products, please visit
www.pacificwoodtech.com/products or call 888.707.2285.



Framing Connectors

FACE MOUNT HANGERS

SINGLE I-JOIST				DOUBLE I-JOIST			
Width	Depth	Hanger	Load ² (100 ³)	Width	Depth	Hanger	Load ² (100 ³)
1 1/2"	9 1/2"	U210 ¹	1215	3"	9 1/2"	MIU3.12/9	2305
	11 7/8"	U210 ¹	1215		11 7/8"	MIU3.12/11	2880
	14"	U210 ¹	1215		14"	MIU3.12/11 ¹	2880
	9 1/2"	IUS1.81/9.5	950		9 1/2"	MIU3.56/9	2305
	11 7/8"	IUS1.81/11.88	1185		11 7/8"	MIU3.56/11	2880
	14"	IUS1.81/14	1420		14"	MIU3.56/14	3170
	16"	IUS1.81/16	1660		16"	MIU3.56/16	3455
	9 1/2"	IUS2.06/9.5	950		9 1/2"	MIU4.28/9	2305
2 1/16"	11 7/8"	IUS2.06/11.88	1185	4 1/8"	11 7/8"	MIU4.28/11	2880
	14"	IUS2.06/14	1420		14"	MIU4.28/14	3170
	16"	IUS2.06/16	1660		16"	MIU4.28/16	3455
	9 1/2"	IUS2.37/9.5	950		9 1/2"	MIU4.75/9	2305
	11 7/8"	IUS2.37/11.88	1185		11 7/8"	MIU4.75/11	2880
	14"	IUS2.37/14	1420		14"	MIU4.75/14	3170
	16"	IUS2.37/16	1660		16"	MIU4.75/16	3455
	18"	MIU2.37/18	3745		18"	MIU4.75/18	3745
2 5/16"	20"	MIU2.37/20	4030	4 5/8"	20"	MIU4.75/20	4030
	22"	MIU2.37/20 ¹	4030		22"	MIU4.75/20 ¹	4030
	24"	MIU2.37/20 ¹	4030		24"	MIU4.75/20 ¹	4030
	9 1/2"	IUS3.56/9.5	1185		9 1/2"	HU410-Z1	2680
	11 7/8"	IUS3.56/11.88	1420		11 7/8"	HU412-Z1	3275
	14"	IUS3.56/14	1420		14"	HU414-Z1	3870
	16"	IUS3.56/16	1660		16"	HU414-Z1 ¹	3870
	18"	MIU3.56/18 ¹	3745		18"	HU414-Z1 ¹	3870
3 1/2"	20"	MIU3.56/20 ¹	4030	7"	20"	HU414-Z1 ¹	3870
	22"	MIU3.56/20 ¹	4030		22"	HU414-Z1 ¹	3870
	24"	MIU3.56/20 ¹	4030		24"	HU414-Z1 ¹	3870
	9 1/2"	IUS3.56/9.5	1185		9 1/2"	See Simpson Catalog*	See Simpson Catalog*
	11 7/8"	IUS3.56/11.88	1420		11 7/8"	See Simpson Catalog*	See Simpson Catalog*
	14"	IUS3.56/14	1420		14"	See Simpson Catalog*	See Simpson Catalog*
	16"	IUS3.56/16	1660		16"	See Simpson Catalog*	See Simpson Catalog*
	18"	MIU3.56/18 ¹	3745		18"	See Simpson Catalog*	See Simpson Catalog*

1. Web stiffeners required.

2. Loads shown are for hangers installed on Douglas-fir-Larch or equivalent. Fill all face nail holes and fill round joist nail holes; Face nails shall be 16d common, except for IUS, which use 10d common.

TOP FLANGE HANGERS

SINGLE I-JOIST				DOUBLE I-JOIST			
Width	Depth	Hanger	Load ² (100 ³)	Width	Depth	Hanger	Load ² (100 ³)
1 1/2"	9 1/2"	ITS1.56/9.5	1520	3"	9 1/2"	LBV3.12/9.5	2590
	11 7/8"	ITS1.56/11.88	1520		11 7/8"	LBV3.12/11.88	2590
	14"	LBV1.56/14	2590		14"	LBV3.12/14	2590
	9 1/2"	ITS1.81/9.5	1520		9 1/2"	MIT49	2305
	11 7/8"	ITS1.81/11.88	1520		11 7/8"	MIT411.88	2305
	14"	ITS1.81/14	1520		14"	MIT414	2305
	16"	ITS1.81/16	1520		16"	MIT416	2305
	9 1/2"	ITS2.06/9.5	1520		9 1/2"	MIT4.28/9.5	2305
2 1/16"	11 7/8"	ITS2.06/11.88	1520	4 1/8"	11 7/8"	MIT4.28/11.88	2305
	14"	ITS2.06/14	1520		14"	MIT4.28/14	2305
	16"	ITS2.06/16	1520		16"	LBV4.28/16	2590
	9 1/2"	ITS2.37/9.5	1520		9 1/2"	MIT359.5-2	2305
	11 7/8"	ITS2.37/11.88	1520		11 7/8"	MIT3511.88-2	2305
	14"	ITS2.37/14	1520		14"	MIT3514-2	2305
	16"	ITS2.37/16	1520		16"	MIT4.75/16	2305
	18"	MIT3518	2305		18"	LBV4.75/18	2590
2 5/16"	20"	MIT3520	2305	4 5/8"	20"	LBV4.75/20	2590
	22"	LBV2.37X	2590		22"	B4.75X	3800
	24"	LBV2.37X	2590		24"	B4.75X	3800
	9 1/2"	ITS3.56/9.5	1520		9 1/2"	B7.12/9.5	3800
	11 7/8"	ITS3.56/11.88	1520		11 7/8"	B7.12/11.88	3800
	14"	ITS3.56/14	1520		14"	B7.12/14	3800
	16"	ITS3.56/16	1520		16"	B7.12/16	3800
	18"	MIT418 ¹	2305		18"	B7.12/18	5650
3 1/2"	20"	MIT420 ¹	2305	7"	20"	B7.12/20	5650
	22"	HIT422	2590		22"	B7.12/22	5650
	24"	HIT424	2590		24"	B7.12/24	5650

1. Web stiffeners required.

2. Loads shown are for hangers installed on Douglas-fir-Larch or equivalent. Fill all face nail holes and fill round joist nail holes; Face nails shall be 16d common, except for ITS, which use 10d common.

ADJUSTABLE HEIGHT HANGERS

SINGLE I-JOIST				DOUBLE I-JOIST			
Width	Depth	Hanger	Load ² (100 ³)	Width	Depth	Hanger	Load ² (100 ³)
1 1/2"	9 1/2"-11 7/8"	THAI222	1715	3"	9 1/2"-14"	THAI-2	2095
1 3/4"	9 1/2"-14"	THAI1.81/22	1715	3 1/2"	9 1/2"-14"	THAI422	1715
2 1/16"	9 1/2"-14"	THAI2.1/2.2	1715	4 1/8"	9 1/2"-14"	THAI-2	2095
2 5/16"	9 1/2"-14"	THAI3.522	1715	4 1/8"	9 1/2"-14"	THAI-2	2095
3 1/2"	9 1/2"-14"	THAI422	1715	7"	9 1/2"-14"	See Simpson Catalog*	See Simpson Catalog*

* Web stiffeners required for all I-joints used with Adjustable Height Hangers.

* For joists 16" and above, See Simpson Wood Construction Connectors catalog for hanger selection.

2. Loads shown use 10d common nails.

VARIABLE PITCH - SINGLE I-JOISTS

SINGLE I-JOIST			
Width	Depth	Hanger	Load (100 ³)
1 1/2"	ALL	VPA2	1050
1 3/4"	ALL	VPA25	1050
2 1/16"	ALL	VPA2.1	1230
2 5/16"	ALL	VPA35	1230
3 1/2"	ALL	VPA4	1230

* VPA connectors provide a bearing length of 2". They should not be used in applications that require longer bearings, such as intermediate supports.

Series	Flange Width
PWI-20	1 3/4"
PWI-45	2 1/16"
PWI-60, PWI-70, PWI-77	2 5/16"
PWI-90	3 1/2"

SKewed 45° HANGERS

SINGLE I-JOIST				DOUBLE I-JOIST			
Width	Depth	Hanger	Load ³ (100 ³)	Width	Depth	Hanger	Load ³ (100 ³)
1 1/2"	9 1/2"	SUR/L210 ¹	1440	3"	9 1/2"	SUR/L210-2 ¹	2015
	11 7/8"	SUR/L210 ¹	1440		11 7/8"	SUR/L210-2 ¹	2015
	14"	SUR/L214	1730		14"	SUR/L214-2 ¹	2500
	9 1/2"	SUR/L1.81/9	1730		9 1/2"	SUR/L1.81/10 ¹	2015
	11 7/8"	SUR/L1.81/11	2305		11 7/8"	SUR/L1.81/12 ¹	2015
	14"	SUR/L1.81/14	2500		14"	SUR/L1.81/15 ¹	2500
	16"	SUR/L1.81/14 ¹	2500		16"	SUR/L1.81/15 ¹	2500
	9 1/2"	SUR/L2.1/9	2015		9 1/2"	HSUR/L4.28/9	1785
2 1/16"	11 7/8"	SUR/L2.1/11	2305	4 1/8"	11 7/8"	HSUR/L4.28/11	2380
	14"	SUR/L2.1/14	2590		14"	HSUR/L4.28/11 ¹	2380
	16"	SUR/L2.1/14	2590		16"	HSUR/L4.28/11 ¹	2380
	9 1/2"	SUR/L2.37/9	2015		9 1/2"	HSUR/L4.75/9	1785
	11 7/8"	SUR/L2.37/11	2305		11 7/8"	HSUR/L4.75/11	2380
	14"	SUR/L2.37/14	2590		14"	HSUR/L4.75/14	2975
	16"	SUR/L2.37/14	2590		16"	HSUR/L4.75/16	3330
	20"	SUR/L2.37/14 ¹	2590		20"	HSUR/L4.75/16 ¹	3330
3 1/2"	9 1/2"	SUR/L410 ¹	2015	7"	9 1/2"	HU410-2X ¹	2145
	11 7/8"	SUR/L410 ¹	2015		11 7/8"	HU412-2X ¹	2620
	14"	SUR/L414 ¹	2500		14"	HU414-2X ¹	3095
	16"	SUR/L414 ¹	2500		16"	HU414-2X ¹	3095
	18"	SUR/L414 ¹	2500		18"	HU414-2X ¹	3095
	20"	SUR/L414 ¹	2500		20"	HU414-2X ¹	3095
	22"	SUR/L414 ¹	2500		22"	HU414-2X ¹	3095
	24"	SUR/L414 ¹	2500				

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